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IN CONFIDENCE TO THE CLIENT

REPORT NO: MT-13/037-B

**LOAD TESTING OF CORONET KWIK-STAGE MODULAR SCAFFOLDS**

CLIENT: CORONET SCAFFOLD GROUP SUZHOU CO., LTD  
SIFC SIP, SUZHOU CITY  
JIANGSU PROVINCE, CHINA  
P: +86-512-85557000 F: +86-512-85557111

DATE OF TEST: APRIL 26<sup>TH</sup> TO JUNE 4<sup>TH</sup> 2013

DATE OF REPORT: JUNE 17<sup>TH</sup> 2013

**TEST SYNOPSIS:**

A Coronet modular scaffold assembly was to be tested by MTS to determine the vertical load carrying capacity of the assembled frame. The modular scaffold system was assembled in accordance with the client’s installation instructions in an enclosed environment at the MTS laboratory. The scope of the test was to determine the maximum limits of the scaffold assembly including the assembled height, number of fully-planked platforms, number of screened platforms and the number of working platforms. Testing was to be conducted in accordance with an approved test procedure conforming to the requirements of AS/NZS 1576.3:1995 SCAFFOLDING, PART 3: PREFABRICATED AND TUBE-AND-COUPLER SCAFFOLDING.

In addition to the aforementioned load testing, MTS was to conduct dimensional analysis and material properties testing of the relevant Coronet scaffolding components to determine the items compliance with AS/NZS 1576.1:2010 - APPENDIX A and AS/NZS 1576.3:1995 - SECTION 2.

**SCAFFOLD IDENTIFICATION:**

Prior to load testing, each individual item used in the scaffold assembly was visually inspected for identification markings. Metal stamped identification marks “Coronet0912” were observed on all components including standards, ledgers, transoms, braces and adjustable jacks.

**TEST PREPARATION:**

Prior to testing, the mass and dimensional attributes of each scaffolding component were recorded. All measurements were conducted using calibrated MTS measuring devices.

The mass of each scaffolding component was used for the determination of the assembled scaffold dead loads (G). The recorded masses and computed dead loads of each individual component are provided in Table 1.



**FIG.1  
CORONET KWIK STAGE  
SCAFFOLD ASSEMBLY**

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AUTHORISED SIGNATORY  
DATE: 17/06/2013**

**DANIEL HUMFREY  
TEST ENGINEER**

### **GEOMETRICAL AND MATERIAL PROPERTIES TESTING:**

Geometrical and material properties tests were conducted for the specific, direct, vertical load bearing scaffolding components as follows:

#### **Standards**

The scaffolding standards were fabricated using ERW steel tube with an outside diameter of 48.3mm (nom.) and a wall thickness of 4.0mm (nom.): **PASS**

The bearing ends of the standards were cut cleanly and square with the axis of the tube: **PASS**

The external surface only of the standards was painted with a 'green' identification colour: **PASS**

Tensile testing as conducted on a test piece machined from the wall of a standard and confirms that the material properties of the standards are as follows:

- Yield Strength of 404MPa: **PASS**
- Ultimate Tensile Strength of 465MPa: **PASS**
- Post-Fracture Elongation of 23%: **PASS**

The material properties of the standard are provided in Appendix B.

#### **Transoms**

The scaffold transoms were fabricated using 50mm x 50mm x 5mm (nom.) angle iron sections. **PASS**

The transoms were painted with a 'green' identification colour: **PASS**

Tensile testing as conducted on a test piece machined from the angle iron material, confirms that the material properties of the transoms are as follows:

- Yield Strength of 327MPa: **PASS**
- Ultimate Tensile Strength of 447MPa: **PASS**
- Post-Fracture Elongation of 29%: **PASS**

The material properties of the transom are provided in Appendix C.

### **SCAFFOLD TEST SET-UP:**

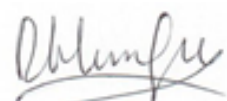
In accordance with AS/NZS 1576.3 Appendix B, the test scaffold was assembled to provide a three (3) bay long by three (3) lift high test assembly. At the height of the second lift, at each end, the test scaffold was rigidly tied to a supporting structure.

A description of the test scaffold assembly is summarised as follows:

<b>Scaffold Height</b>	6.5m, eg. Three (3) x 2.0m lifts plus Jacks.
<b>Scaffold Length</b>	7.2m, eg. Three (3) x 2.4m bays
<b>Scaffold Width</b>	1.2m (nom.)
<b>Standards</b>	1.0m (nom.), 2.0m (nom.) and 3.0m (nom.), assembled with staggered joints.
<b>Transoms</b>	Located at 2.0m (nom.) elevations, fitted between lateral standards.
<b>Ledgers</b>	Located at 2.0m (nom.) elevations, on both sides of the structure.
<b>Face Bracing</b>	One brace per 2.7m (nom.) elevation, located every 3 <sup>rd</sup> bay.
<b>Lateral Bracing</b>	Fitted at both ends of the test scaffold.
<b>Planks</b>	Five (5) x 2.4m planks on the 3 <sup>rd</sup> lift of the 2 <sup>nd</sup> bay.
<b>Screw Jacks</b>	Fitted in the fully extended position of 480mm.



**ROD WILKIE**  
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**DATE: 17/06/2013**



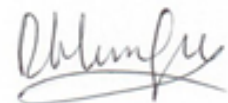
**DANIEL HUMFREY**  
**TEST ENGINEER**

Scaffold Item	Length of Item (mm)	Mass Per Item (kg)	Comments
Standard 0.99m	990	6.4	1m (nom.) standard with spigot, 48.3mm outside diameter, 4.0mm wall thickness
Standard 1.98m	1980	11.2	2m (nom.) standard with spigot, 48.3mm outside diameter, 4.0mm wall thickness
Standard 2.97m	2970	16.8	3m (nom.) standard with spigot, 48.3mm outside diameter, 4.0mm wall thickness
Ledger 2.4m	2400	9.2	2.4m (nom.) ledger, 48.3mm outside diameter, 3.2mm wall thickness
Ledger 1.8m	1800	6.8	1.8m (nom.) ledger, 48.3mm outside diameter, 3.2mm wall thickness
Ledger 1.2m	1200	5.0	1.2m (nom.) ledger, 48.3mm outside diameter, 3.2mm wall thickness
Transom 1.2m	1200	9.6	1.2m (nom.) transom, 50mm width, 4.7mm angle line thickness
Transom 1.8m	1800	16.0	1.8m (nom.) transom, 50mm width, 4.7mm angle line thickness
Transom 2.4m	2400	21.4	2.4m (nom.) transom, 50mm width, 4.7mm angle line thickness
Diagonal Face Brace 3.52m	3520	13.6	3.5m (nom.) brace , 48.3mm outside diameter, 3.2mm wall thickness
Lateral Brace 2.4m	2400	9.4	2.4m (nom.) brace, 48.3mm outside diameter, 3.2mm wall thickness
Plank 2.4m	2400	14.2	2.4m (nom.) plank, pressed metal with perforations. Explicit dimensional details in MTS report: MT-13-037-A1
Plank 1.8m	1800	11.4	1.8m (nom.) plank, pressed metal with perforations. Explicit dimensional details in MTS report: MT-13-037-A2
Plank 1.2m	1200	7.9	1.2m (nom.) plank, pressed metal with perforations. Explicit dimensional details in MTS report: MT-12-037-A3
Toe Board 2.4m	2400	14.2	2.4m plank used as toe board.
Two Board Hop-Up Bracket	N/A	6.0	33.5mm outside diameter, 4.7mm angle line thickness
Three Board Hop-Up Bracket	N/A	10.2	33.5mm outside diameter, 4.7mm angle line thickness
End Toe board 1.2m	1200	7.9	1.2m plank used as toe board.
<i>Aluminium Stair &amp; Handrail</i>	<i>2400</i>	<i>46.0</i>	<i>Nominal mass from typical industry components</i>
<i>Infill Panels</i>	<i>2400</i>	<i>14.6</i>	<i>Nominal mass from typical industry components</i>
<i>Infill Panels</i>	<i>2400</i>	<i>14.2</i>	<i>Nominal mass from typical industry components</i>

**TABLE.1**  
**SCAFFOLD COMPONENTS**



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**TEST ENGINEER**



**LOAD TESTING:**

Load testing was to be conducted on two laterally adjacent standards referred to as B1 and B2 in this report. The loaded standards were located at the third pair of laterally adjacent standards or bay 3 of the test structure (see Fig.2). Transoms and ledgers, placed at two metre height intervals, were used to connect laterally adjacent standards and longitudinally adjacent standards respectively.

**SCAFFOLD TEST SCENARIOS:**

In consultation with the client, a range of test scenarios were engineered in order to achieve a staged increase in test load. Each stage was commensurate with a nominated scaffold height and an interval number of working platforms.

All load tests were conducted using dead loads commensurate for a fully-planked at 2.0 metre intervals, ledgers installed at 0.5m spacing on the outer side and ledgers installed at 1.0m spacing on the inner (building) side scaffold assembly. Additionally the scaffold was assumed to incorporate gravity loads from an adjoining stairway access structure. Furthermore, gravity loads from associated safety screens and an aluminium access stairway structure were assumed to be applied symmetrically over the scaffold. Load tests were conducted for scaffolding assemblies using both single and double working platform scenarios.



**FIG.2  
SCAFFOLD UNDER PEAK TEST  
LOAD**

**CALCULATION OF TEST LOADS:**

Test loads B1 and B2 were calculated in accordance with AS/NZS 1576.1 and AS/NZS 1576.3 - Appendix B. Dead loads “G” were calculated from the test data provided in Table.1. A live load “Q” of 6.6kN, corresponding to a heavy duty scaffold was adopted for the tests.

**Dead Load Calculations (G):**

**Test Scenario 1**

Scaffold height: 10m  
 No. of Working Platforms: 1  
 G = 989kg or 9.7kN  
 W= 354kg or 3.5kN

**Test Scenario 2**

Scaffold height: 16m  
 No. of Working Platforms: 1  
 G = 1672kg or 16.4kN  
 W= 354kg or 3.5kN

**Test Scenario 3**

Scaffold height: 10m  
 No. of Working Platforms: 2  
 G = 989kg or 9.7kN  
 W= 354kg or 3.5kN

**Test Loads (B1 & B2):**

**Test Scenario 1**

Scaffold height: 10m  
 No. of Working Platforms: 1  
**B1 = 18.9kN**  
**B2 = 13.7N**

**Test Scenario 2**

Scaffold height: 16m  
 No. of Working Platforms: 1  
**B1 = 25.6kN**  
**B2 = 20.4kN**

**Test Scenario 3**

Scaffold height: 10m  
 No. of Working Platforms: 2  
**B1 = 28.2kN**  
**B2 = 17.6kN**

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**Dead Load Calculations (G):**

**Test Scenario 4**

Scaffold height: 20m  
 No. of Working Platforms: 1  
 G = 2130kg or 20.9kN  
 W = 354kg or 3.5kN

**Test Scenario 5**

Scaffold height: 24m  
 No. of Working Platforms: 1  
 G = 2589kg or 25.4kN  
 W = 354kg or 3.5kN

**Test Scenario 6**

Scaffold height: 26m  
 No. of Working Platforms: 1  
 G = 2813kg or 27.6kN  
 W = 354kg or 3.5kN

**Test Scenario 7**

Scaffold height: 20m  
 No. of Working Platforms: 2  
 G = 2130kg or 20.9kN  
 W = 354kg or 3.5kN

**Live Load Calculations (Q):**

**Test Scenario 1**

Scaffold height: 10m  
 No. of Working Platforms: 1  
 Q = 9.2kN Standard B1  
 Q = 4.0kN Standard B2

**Test Scenario 2**

Scaffold height: 16m  
 No. of Working Platforms: 1  
 Q = 9.2kN Standard B1  
 Q = 4.0kN Standard B2

**Test Scenario 3**

Scaffold height: 10m  
 No. of Working Platforms: 2  
 Q = 18.5kN Standard B1  
 Q = 7.9kN Standard B2

**Test Scenario 4**

Scaffold height: 20m  
 No. of Working Platforms: 1  
 Q = 9.2kN Standard B1  
 Q = 4.0kN Standard B2

**Test Loads (B1 & B2):**

**Test Scenario 4**

Scaffold height: 20m  
 No. of Working Platforms: 1  
**B1 = 30.1kN**  
**B2 = 24.9kN**

**Test Scenario 5**

Scaffold height: 24m  
 No. of Working Platforms: 1  
**B1 = 34.6kN**  
**B2 = 29.4kN**

**Test Scenario 6**

Scaffold height: 26m  
 No. of Working Platforms: 1  
**B1 = 36.8kN**  
**B2 = 31.6kN**

**Test Scenario 7**

Scaffold height: 20m  
 No. of Working Platforms: 2  
**B1 = 39.4kN**  
**B2 = 28.8kN**

**Live Load Calculations (Q):**

**Test Scenario 5**

Scaffold height: 24m  
 No. of Working Platforms: 1  
 Q = 9.2kN Standard B1  
 Q = 4.0kN Standard B2

**Test Scenario 6**

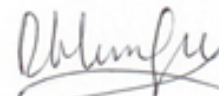
Scaffold height: 26m  
 No. of Working Platforms: 1  
 Q = 9.2kN Standard B1  
 Q = 4.0kN Standard B2

**Test Scenario 7**

Scaffold height: 20m  
 No. of Working Platforms: 2  
 Q = 18.5kN Standard B1  
 Q = 7.9kN Standard B2



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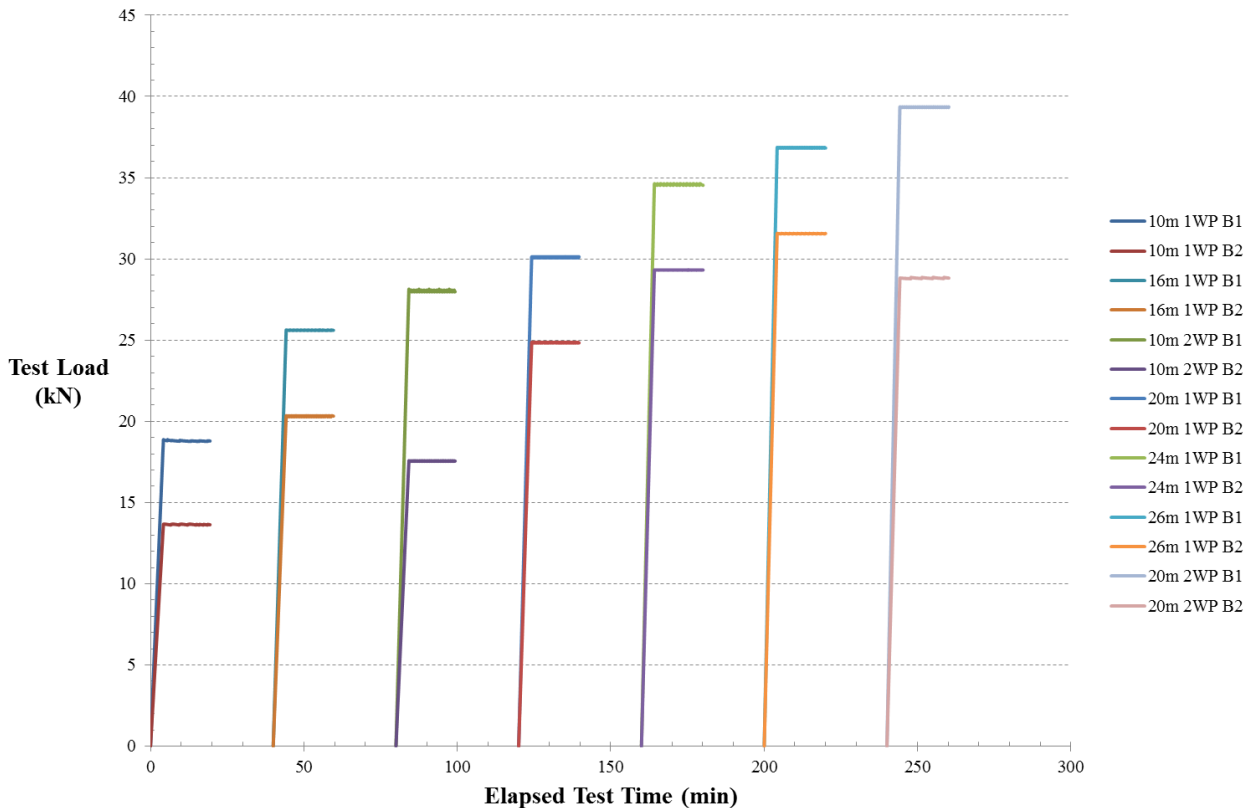


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 TEST ENGINEER

**LOAD TEST METHOD:**

Load testing was conducted using an MTS loading beam mounted on top of the spigot ends of the adjacent standards referred to as B1 and B2. Test load was progressively applied to the loading beam using two hydraulic cylinders positioned symmetrically on the outer side of B1 and B2. Force measuring devices were located between each hydraulic cylinder and the relative connection points to the loading beam (see Fig.2).

Testing was conducted by progressively applying the relevant computed test loads, B1 and B2, for specific scaffold assembly erection scenarios. The peak test loads, B1 and B2, were maintained for a period of 15 minutes. The applied test load, B1 and B2, versus the elapsed test time for each loading scenario is provided in Fig.3.



**TEST LOAD VS ELAPSED TIME DATA**

**TEST OBSERVATIONS AND COMMENTS:**

The scaffold structure successfully maintained the applied test loads B1 and B2 for each test scenario (refer to the calculation of test loads section of the report) for the specified period of 15 minutes. A peak dynamic lateral deflection of 21mm was recorded in the loaded standards when subjected to loading conditions commensurate with test scenario No.7. Upon release of the test load, the loading standards were noted to have rebounded and there was no sign of permanent yielding or residual deformation in either of the test standards.

**TEST FACILITY:**

All testing was carried-out by test engineers Rod Wilkie and Daniel Humfrey in an enclosed environment at the MTS laboratory. Testing was conducted between April 26<sup>th</sup> and June 4<sup>th</sup> 2013.

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**SUMMARY:**

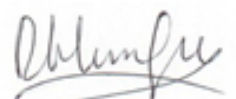
Dimensional analysis and material properties testing confirms that the relevant Coronet scaffolding components have met the specific requirements for AS/NZS 1576.1:2010 - Appendix A – Clause A5. Furthermore, the aforementioned testing regime verifies that the relevant scaffolding components met the specific requirements for AS/NZS 1576.3:1995 – Section 2 – Clause 2.4.1, Clause 2.4.2 & Clause 2.4.3.1.

Testing as described and reported herein verifies that the Coronet kwik-stage modular scaffold has met the performance requirements for AS/NZS 1576.2:1995 - Appendix B for the specific test scenarios provided in this report. It is important to note that this test report is limited to the installation procedures and the specific scaffolding components (see Table 1) as used during load testing of the test structure.

The partially planked and screened assembly scenarios provided in Appendix A, are based on the test loads reported herein. The partially planked and screened assembly scenarios are specific to the installation procedures and the specific scaffolding components (see Table 1) as used during load testing of the test structure.



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**FULLY PLANKED SCAFFOLD ASSEMBLY SCENARIOS  
(PLANKED AND SCREENED EVERY TWO (2) METRES)**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>10 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>5</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>1</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>4</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>5</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE 2A  
SINGLE WORKING PLATFORM**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>16 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>8</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>1</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>7</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>8</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE 2B  
SINGLE WORKING PLATFORM**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>10 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>5</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>2</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>3</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>5</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE 2C  
TWO WORKING PLATFORMS**

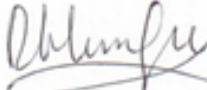
<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>20 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>10</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>1</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>9</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>10</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE 2D  
SINGLE WORKING PLATFORM**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>24 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>12</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>1</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>11</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>12</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE 2E  
SINGLE WORKING PLATFORM**

  
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 DANIEL HUMFREY  
 TEST ENGINEER

**FULLY PLANKED SCAFFOLD ASSEMBLY SCENARIOS  
(PLANKED AND SCREENED EVERY TWO (2) METRES)**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>26 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>13</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>1</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>12</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>13</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE 2F  
SINGLE WORKING PLATFORM**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>20 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>10</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>2</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>8</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>10</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE 2G  
TWO WORKING PLATFORM**

Notes:

1. Melbourne Testing Services Pty Ltd shall not be liable for loss, cost, damages or expenses incurred by the client or any other person or company, resulting from the use of any information or interpretation given in this report. In no case shall Melbourne Testing Services Pty Ltd be liable for consequential damages including, but not limited to, lost profit, damages for failure to meet deadlines and lost production arising from this report. This document shall not be reproduced except in full and relates only to the items tested.
2. This report only indicates compliance of the scaffold assembly in its state at the time of testing. It should not be taken as a statement that all similar scaffolds or components of scaffolds in all states of repair, would also be found to comply.
3. It remains the responsibility of the client to ensure that the scaffolding components as reported herein are representative of the entire production batch.
4. As described and reported herein, the load capacity of the Coronet kwik-stage modular scaffold assembly is specific to the requirements of AS/NZS 1576.3:1995 Appendix B.
5. MTS shall take no responsibility for the load performance attributes of scaffold assemblies which are erected other than as specifically described under the heading "Scaffold Test Scenarios" on Page 3 of the MTS test report (MT-13/037-B).
6. MTS shall take no responsibility for the load performance of scaffold assemblies constructed from scaffolding components other than those specifically described in Table.1 of the MTS test report (MT-13/037-B).
7. The load performance attributes of partially planked and screened scaffold assemblies, as described in Appendix A of the MTS test report (MT-13/037-B), are specific to scaffolding components as described in Table 1.
8. MTS shall take no responsibility for the procurement and authenticity of the scaffold as described herein.
9. MTS shall take no responsibility for the installation procedures used for the scaffold described herein.



**RODNEY WILKIE  
AUTHORISED SIGNATORY  
DATE: 17/06/2013**



**DANIEL HUMFREY  
TEST ENGINEER**

**APPENDIX A****PARTIALLY PLANKED AND SCREENED SCAFFOLD ASSEMBLY SCENARIOS**

The test loads required for partially planked and screened scaffold assembly scenarios were computed from appropriate dead (G) and live (Q) loads for the specific structure. MTS has provided partially planked and screened scaffold assembly scenarios, where the total magnitude of factored dead (G) load and live (Q) load is equal to or less than the maximum test load achieved on the tested scaffold assembly.

The computed theoretical loads for standards B1 and B2 are nominally equal to or less than the peak test loads applied to the test scaffold and are as follows:

- 26m high, two (2) working platform, planked every 4m. **B1=35.0kN, B2=24.5kN**
- 30m high, single working platform, planked every 4m. **B1=31.8kN, B2=26.5kN**
- 30m high, two (2) working platform, planked every 8m. **B1=36.2kN, B2=25.7kN**
- 36m high, single working platform, planked every 6m. **B1=32.8kN, B2=27.5kN**

**26M PARTIALLY PLANKED AND SCREENED, TWO (2) WORKING PLATFORMS**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>26 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>5</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>2</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>3</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>5</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE A1.**  
**TWO (2) WORKING PLATFORMS**

**30M PARTIALLY PLANKED AND SCREENED, SINGLE WORKING PLATFORM**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>30 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>8</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>1</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>7</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>8</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE A2.**  
**SINGLE WORKING PLATFORM**

**30M PARTIALLY PLANKED AND SCREENED, TWO (2) WORKING PLATFORMS**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b>30 METRES</b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b>4</b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b>2</b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b>2</b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b>4</b>
<i>MAXIMUM JACK HEIGHT</i>	<b>480MM</b>

**TABLE A3.**  
**TWO (2) WORKING PLATFORMS**

**36M PARTIALLY PLANKED AND SCREENED**

<i>MAXIMUM ASSEMBLED HEIGHT</i>	<b><i>36 METRES</i></b>
<i>MAXIMUM NUMBER OF PLANKED AND SCREENED PLATFORMS</i>	<b><i>6</i></b>
<i>MAXIMUM NUMBER OF WORKING PLATFORMS</i>	<b><i>1</i></b>
<i>NUMBER OF CLOSED PLATFORMS</i>	<b><i>5</i></b>
<i>MAXIMUM NUMBER OF HOP UPS</i>	<b><i>6</i></b>
<i>MAXIMUM JACK HEIGHT</i>	<b><i>480MM</i></b>

**TABLE A4.  
SINGLE WORKING PLATFORM**

**APPENDIX B:**



Melbourne Testing Services Pty Ltd  
 ABN: 11 088 395 153  
**Delivery Address:**  
 1/15 Pickering Road  
 Mulgrave Vic 3170  
 Telephone:  
 Facsimile:  
 Email Address:  
 Web Address:

**Postal Address:**  
 PO Box 5111  
 Brandon Park Vic 3150  
 61 3 9560 2759  
 61 3 9560 2769  
 info@melbtest.com.au  
 www.melbtest.com.au

**Tensile Test Report**

**Report No:** MT-13-037-B  
**Report Date:** 11-Jun-13  
**Specimen I.D.:** Scaffold Standard  
**Testing Machine:** TE 20854

**TEST DETAILS**

Test Date: *11/06/2013*  
 Extensometer Gauge Length:  $L_e$  (mm) 

50.00
-------

**SPECIMEN DETAILS**

Width:  $b$  (mm) 

12.31
-------

  
 Thickness:  $a$  (mm) 

3.78
------

  
 Area:  $S_o$  (mm<sup>2</sup>) 

47.14
-------

  
 Gauge Length:  $L_o$  (mm) 

40.00
-------

  
 Parallel Length:  $L_c$  (mm) 

75.00
-------

**TENSILE PROPERTIES**

Tensile Strength:  $R_m$  (MPa) 

465
-----

  
 Proof Stress:  $R_{p0.2}$  (MPa) 

404
-----

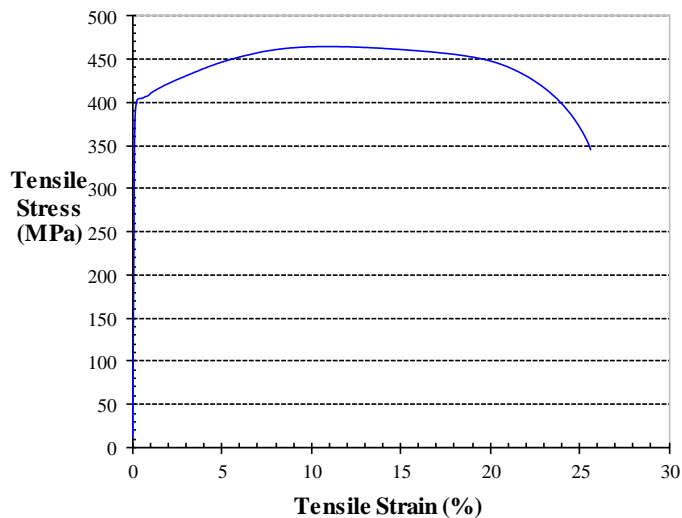
  
 Post Fracture Elongation:  $A$  (%) 

23
----

**Test Comments:**

Tested in accordance with AS 1391-2007. Area calculated in accordance with AS 1391 Appendix D.

Ben McPhee  
**Authorised Signatory**



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**Accreditation No: 1047**



**APPENDIX C:**



Melbourne Testing Services Pty Ltd  
 ABN: 11 088 395 153  
**Delivery Address:**  
 1/15 Pickering Road  
 Mulgrave Vic 3170  
 Telephone:  
 Facsimile:  
 Email Address:  
 Web Address:

**Postal Address:**  
 PO Box 5111  
 Brandon Park Vic 3150  
 61 3 9560 2759  
 61 3 9560 2769  
 info@melbtest.com.au  
 www.melbtest.com.au

**Tensile Test Report**

**Report No:** MT-13-037-B  
**Report Date:** 11-Jun-13  
**Specimen I.D.** Scaffold Transom  
**Testing Machine:** TE 20854

**TEST DETAILS**

Test Date: **11/06/2013**  
 Extensometer Gauge Length:  $L_e$  (mm) 

50.00
-------

**SPECIMEN DETAILS**

Width:  $b$  (mm) 

12.29
-------

  
 Thickness:  $a$  (mm) 

4.51
------

  
 Area:  $S_o$  (mm<sup>2</sup>) 

55.43
-------

  
 Gauge Length:  $L_o$  (mm) 

50.00
-------

  
 Parallel Length:  $L_c$  (mm) 

75.00
-------

**TENSILE PROPERTIES**

Tensile Strength:  $R_m$  (MPa) 

447
-----

  
 Proof Stress:  $R_{p0.2}$  (MPa) 

327
-----

  
 Upper Yield Stress:  $R_{eH}$  (MPa) 

336
-----

  
 Lower Yield Stress:  $R_{eL}$  (MPa) 

334
-----

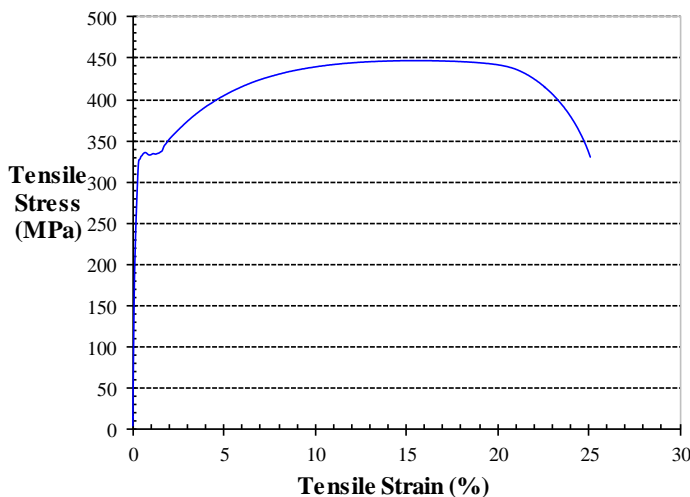
  
 Post Fracture Elongation:  $A$  (%) 

29
----

**Test Comments:**

Tested in accordance with AS 1391-2007.

Ben McPhee  
**Authorised Signatory**



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